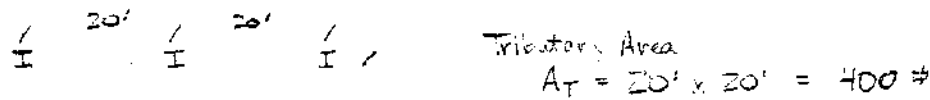


3/28/06

- 1.11 A 3 story hotel has interior columns that are spaced 20' apart in two perpendicular directions. If the loading on the flat roof is estimated to be 30 lb/ft^2 , determine the live load supported by a typical interior column at (a) the ground-floor level, & (b) the second-floor level.



$$L = L_0 \left(0.25 + \frac{15}{\sqrt{K_{LL} A_T}} \right)$$

$$L_0 = 40 \text{ - Table 1-4}$$

Great job!
 (Prob 12 graded separately)

$$K_{LL} = 4 \text{ for interior columns}$$

a) $A_T = 400 \text{ ft}^2$

$$F_G = (30 \text{ lb/ft}^2)(400 \text{ ft}^2) = 12,000 \text{ lb} = 12 \text{ kip}$$

$$L = 40 \left(0.25 + \frac{15}{\sqrt{4(400)}} \right) = 25 \text{ lb/ft}^2 \quad (1000 \text{ ft}^2 > 400 \text{ ft}^2)$$

$$F_1 = (25 \text{ lb/ft}^2)(400 \text{ ft}^2) = 10,000 \text{ lb} = 10 \text{ kip}$$

$$F_2 = (25 \text{ lb/ft}^2)(400 \text{ ft}^2) = 10,000 \text{ lb} = 10 \text{ kip}$$

$$F = F_G + F_1 + F_2 = 12 + 10 + 10$$

$$F = 32.0 \text{ kip}$$

b) $A_T = 400 \text{ ft}^2$

$$F_G = (30 \text{ lb/ft}^2)(400 \text{ ft}^2) = 12,000 \text{ lb} = 12 \text{ kip}$$

$$L = 40 \left(0.25 + \frac{15}{\sqrt{4(400)}} \right) = 25 \text{ lb/ft}^2$$

$$F_1 = (25 \text{ lb/ft}^2)(400 \text{ ft}^2) = 10,000 \text{ lb} = 10 \text{ kip}$$

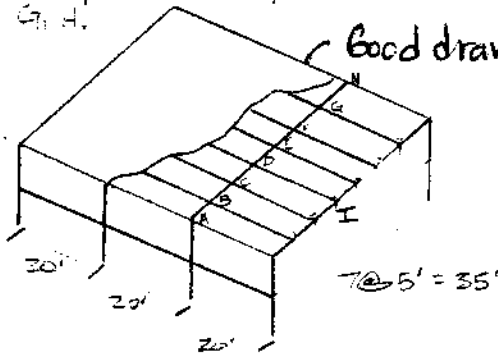
$$F = F_G + F_1 = 12 + 10$$

$$F = 22.0 \text{ kip}$$

8,22/16

2.1 The roof deck of the single story building is subjected to a dead plus live load $w = 95 \text{ lb/ft}^2$. If the purlins are spaced 5' & the bents are spaced 20' apart, determine the distributed loading that acts along the purlin DI, & the loadings that act on the bent @ A, B, C, D, E, F, G, H.

8/10



Good drawing!

Tributary area for bents B-G: $5' \times 20' = 100 \text{ ft}^2$

$$F = (95 \text{ lb/ft}^2)(100 \text{ ft}^2)$$

$$F = \cancel{9500 \text{ lb}} = F_{B-G}$$

$$\downarrow 4750 \text{ lb}$$

Tributary area for bents A & H:

$$A_{A,H} = 2.5' \times 20' = 50 \text{ ft}^2$$

$$F_{A,H} = (95 \text{ lb/ft}^2)(50 \text{ ft}^2) = 4750 \text{ lb}$$

$$\boxed{F_{A,H} = \cancel{4750 \text{ lb}} = 2375 \text{ lb}}$$

You must divide by two because one-half of the load is on bent I and one-half is on bent II.

Tributary area for DI:

$$A_T = 5' \times 20' = 100 \text{ ft}^2$$

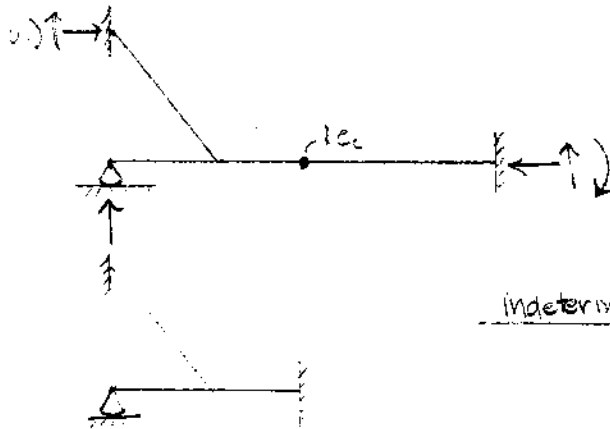
$$F_{DI} = (95 \text{ lb/ft}^2)(100 \text{ ft}^2) = 9500 \text{ lb}$$

$$F_{\text{along DI}} = 9500 \text{ lb} / (20 \text{ ft})$$

$$\boxed{F_{\text{along DI}} = 475 \text{ lb/ft}} \quad \checkmark$$

3/28/06

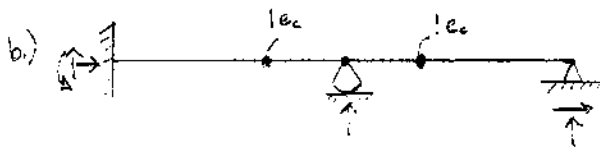
29 Classify each of the structures as statically determinate, statically indeterminate, or unstable. If indeterminate, specify the degree of indeterminacy.



6 unknowns > 3 eqns + 1e_c = 4 equations

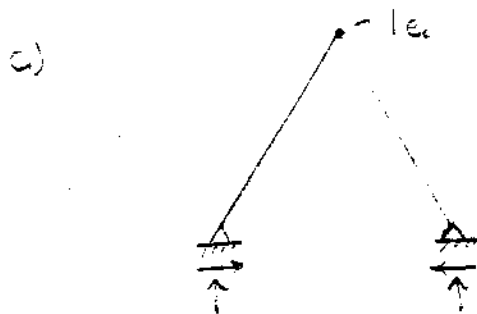
indeterminate \Rightarrow unstable

indeterminate to 2°



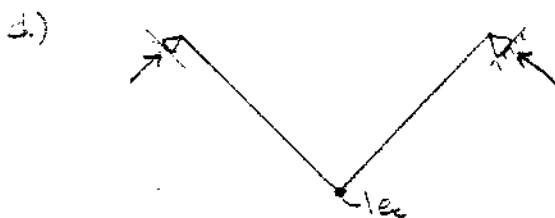
6 unknowns > 3 eqns + 2e_c = 5 equations

indeterminate to 1°



4 unknowns = 4 equations

Stable: statically determinate

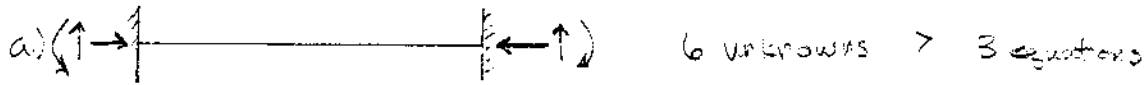


2 unknowns < 4 equations

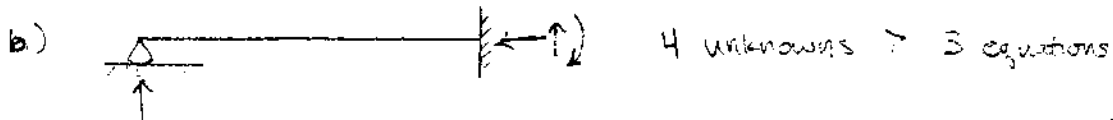
\therefore unstable

10
10

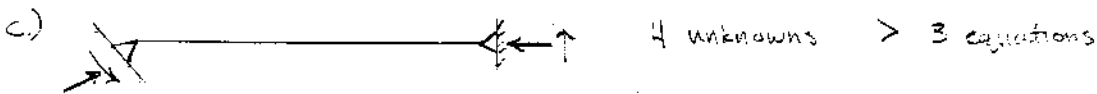
2.11 Classify each of the structures as statically determinate, indeterminate, or unstable. If indeterminate specify the degree of indeterminacy. The supports & connections are to be assumed as stated.



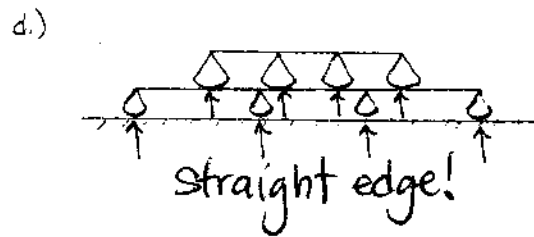
∴ indeterminate to 3° ✓



∴ indeterminate to 1° ✓



∴ indeterminate to 1° ✓

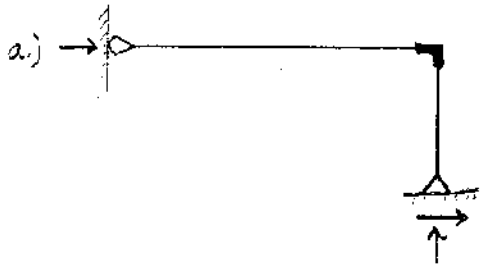


unstable - all reactions are parallel ✓

8/28/06

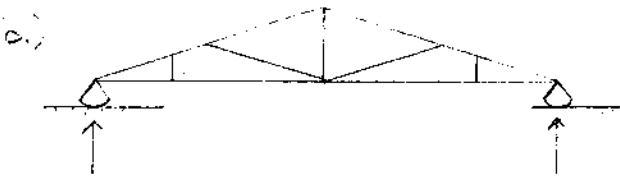
2.15 Classify each of the structures as statically determinate, indeterminate, or unstable. If indeterminate, specify the degree of indeterminacy. The supports or connections are to be assumed as stated.

9/10

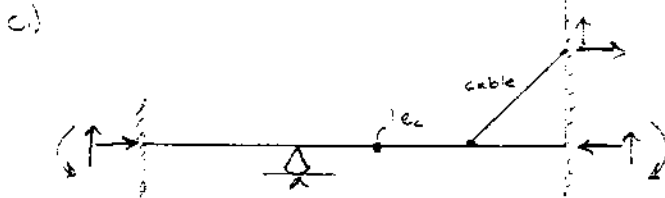


3 unknowns = 3 equations

stable ∴ determinate ✓



parallel reaction ∴ unstable ✓

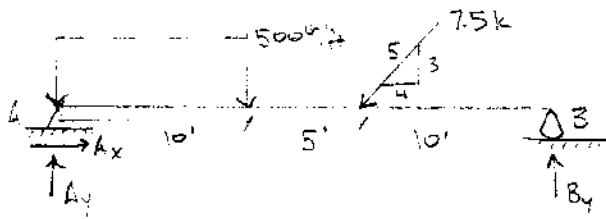


9 unknowns > 3 + 1 = 4 equations

stable ∴ indeterminate to 5th order

-1

2.13 Determine the reactions on the beam. The support at B can be assumed as a roller.



3 unknowns = 3 equations

stable \therefore determinate

$$\begin{aligned}\Sigma M_A = 0 &= -500 \text{ lb/ft} (10 \text{ ft}) \left(\frac{10}{2}\right) - \frac{3}{5} (7.5) (15) + B_y (25) \\ 0 &= -25,000 \text{ lb} - 67,500 \text{ lb} + 25 B_y\end{aligned}$$

$$B_y = 3700 \text{ lb} = 3.7 \text{ kip} \quad \checkmark$$

$$\Sigma F_y = 0 = -500 (10) - \frac{3}{5} (7.5) + B_y + A_y$$

$$A_y = 5300 \text{ lb} = 5.3 \text{ kip} \quad \checkmark$$

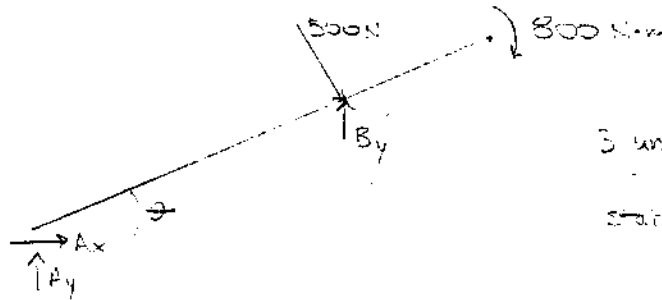
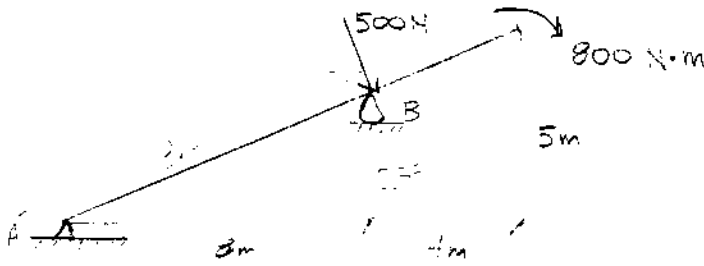
$$\Sigma F_x = 0 = A_x - \frac{4}{5} (7.5)$$

$$A_x = 6 \text{ kip} \quad \checkmark$$

10/6

2.22 Determine the reactions at the supports.

10
10



3 unknowns = 3 equations
stable & determinate

$$\tan \theta = \frac{3}{4}$$

$$\theta = 22.62^\circ$$

$$\sum M_A = 0 = -500(3.67) + \frac{3}{5} B_y (5) - 800$$

$$B_y = 641.9 \text{ N} \quad \checkmark$$

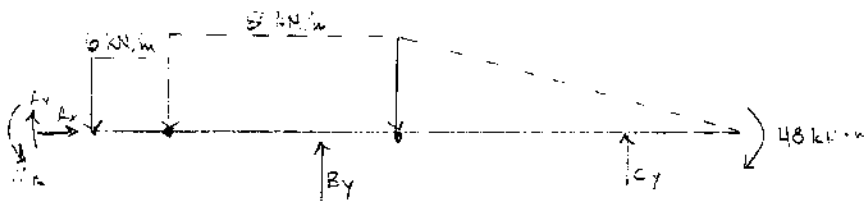
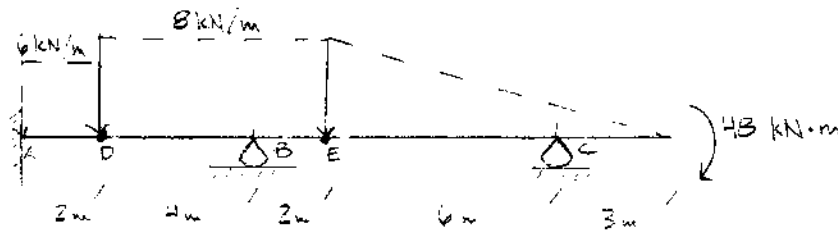
$$\sum F_y = 0 = -500 \left(\frac{3}{5}\right) + 642 + A_y$$

$$A_y = 180 \text{ N} \quad \checkmark$$

$$\sum F_x = 0 = A_x + 500 \sin 22.62$$

$$A_x = -192 \text{ N or } 192 \text{ N} \leftarrow \quad \checkmark$$

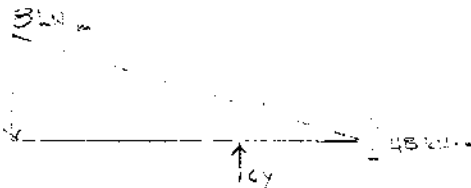
The compound beam is fixed supported at A & supported by rollers at B & C. If there are moments @ D & E, determine the reactions at the supports A, B, & C.



3 unknowns = 3 eq equations
statically determinate

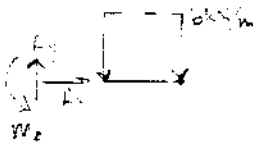
$$\sum F_x = 0 = A_x$$

$$A_x = 0 \text{ N}$$



$$\sum M_E = 0 = -\frac{1}{2} \cdot 8 \cdot (2) \cdot \left(\frac{2}{3} \cdot 2\right) - 48 \cdot 6 + 6 C_y$$

$$C_y = 26 \text{ kN}$$



$$\sum M_D = 0 = M_A + 6 \cdot (2) \cdot \left(\frac{2}{2}\right) - 70 + A_y \cdot 2$$

$$A_y = \frac{M_A + 12}{2}$$

(1)

$$\sum F_y = 0 = -6 \cdot (2) - 8 \cdot (6) - \frac{1}{2} \cdot (8 \cdot 9) + A_y + B_y + C_y$$

$$A_y = 70 - B_y$$

(2)

$$\sum M_A = 0 = -6 \cdot (2) \cdot (1) - 8 \cdot (6) \cdot (5) - \frac{1}{2} \cdot 8 \cdot (9) \cdot (14) - 48 \cdot 6 + 26 \cdot (14) + B_y \cdot (6) + M_A$$

$$304 = 6B_y + M_A$$

(3)

$$(1) \rightarrow (2): \frac{M_A + 12}{2} = 70 - B_y$$

$$M_A = 2(70 - B_y) - 12$$

(4)

$$(4) \rightarrow (3): 304 = 6B_y + [2(70 - B_y) - 12]$$

$$304 = 6B_y + 140 - 2B_y - 12$$

$$B_y = 45 \text{ N}$$

(5)

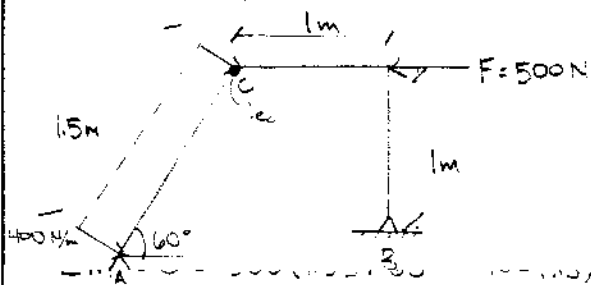
$$(5) \rightarrow (2): A_y = 70 - 45$$

$$A_y = 25 \text{ N}$$

$$(5) \rightarrow (1): 25 = \frac{M_A + 12}{2}$$

$$M_A = 38 \text{ kNm}$$

2.44 Determine the horizontal & vertical components of force that pins A & B exert on the two member frame. Set $F = 500\text{ N}$



$$(1) \rightarrow (2): 0 = 629.5 - 450 + 0.294 B_x + 1.75 B_y$$

$$B_x = -97.4 \text{ N or } 97.4 \text{ N } \rightarrow \quad (3)$$

$$(5) \rightarrow (4): B_y = -97.4 \text{ N or } 97.4 \text{ N } \downarrow$$

$$\Sigma F_x = 0 = A_x + 400(1.5) \sin 60^\circ - B_x - 500$$

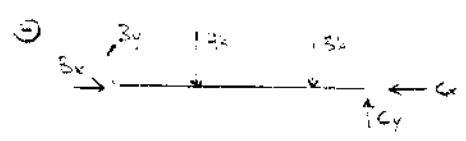
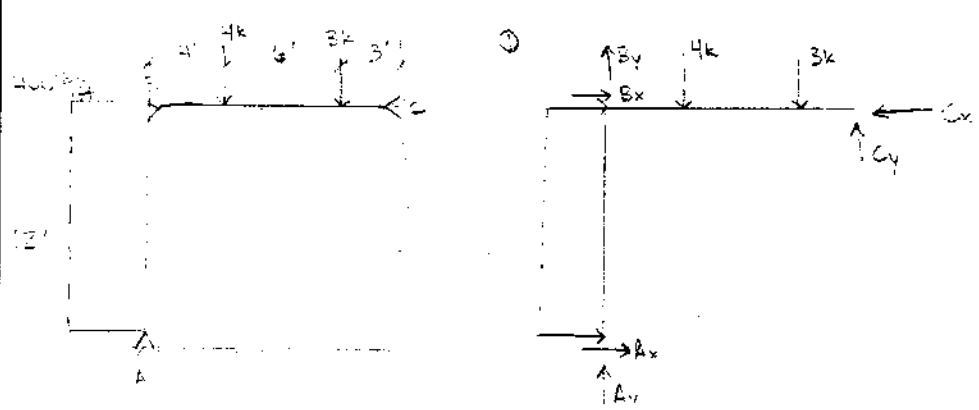
$$0 = A_x + 520 - (-97.4) - 500$$

$$A_x = -117.4 \text{ N or } 117.4 \text{ N } \leftarrow$$

$$\Sigma F_y = 0 = -400(1.5) \cos 60^\circ + A_y + B_y$$

$$A_y = 200 \text{ N}$$

2.5 Determine the horizontal & vertical reactions & force at the connections A, B, & C. Assume each of these connections is a pin.

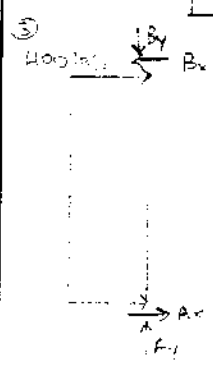


② $\sum M_B = 0 = -4000(4) - 3000(10) + C_y(13)$

$C_y = 3538.5 \text{ lb} = 3.54 \text{ kip}$

③ $\sum F_y = 0 = -4000 - 3000 + 3538.5 + B_y$

$B_y = 3461.5 \text{ lb} = 3.46 \text{ kip}$



④ $\sum F_y = 0 = -B_y + A_y$
 $0 = -3461.5 + A_y$

$A_y = 3.46 \text{ kip}$

⑤ $\sum M_B = 0 = 400(12)(\frac{12}{2}) + A_x(12)$

$A_x = -2400 \text{ lb or } 2.40 \text{ kip} \leftarrow$

$A_x = 2.40 \text{ kip} \leftarrow$

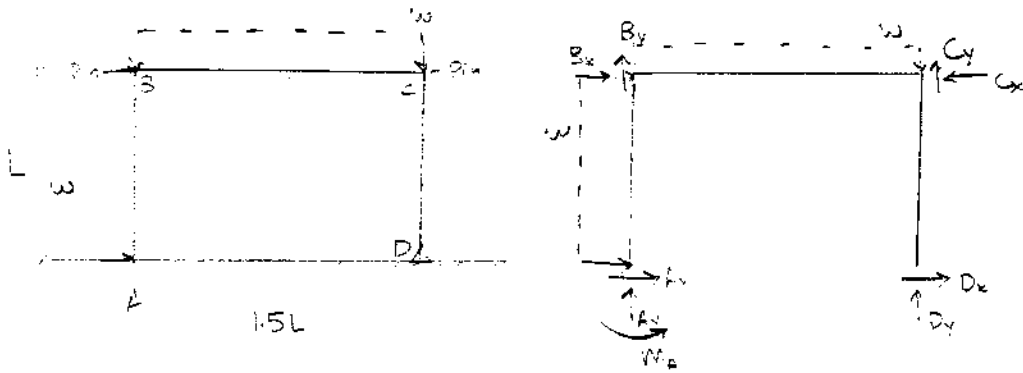
⑥ $\sum F_x = 0 = -B_x + A_x$

$B_x = 2.40 \text{ kip} \leftarrow$

⑦ $\sum M_A = 0 = 2.4(12) - 4(4) - 3(10) + 3.54(13) + C_x(12)$

$C_x = 2.40 \text{ kip}$

2.46 Determine the reactions at the supports A & D. Assume A is fixed, and B & C & D are pins.



Member CD

$$\sum M_D = 0 = C_x(L)$$

$$C_x = 0$$

$$\sum F_x = 0 = -C_x + D_x$$

$$D_x = 0$$

using (1)

$$\sum F_y = 0 = -0.75wL + D_y$$

$$D_y = 0.75wL$$

Member BC

$$\sum F_x = 0 = -C_x + B_x$$

$$B_x = 0$$

$$\sum M_C = -1.5Lw(\frac{1.5L}{2}) + C_y(1.5L)$$

$$C_y = 0.75wL \quad (1)$$

$$\sum F_y = 0 = -1.5Lw + 0.75w + B_y$$

$$B_y = 1.5Lw - 0.75w$$

Member AB

$$\sum F_x = 0 = B_x - A_x + wL$$

$$-A_x = wL$$

or

$$A_x = wL \leftarrow$$

$$\sum M_A = 0 = -B_x(L) - wL(\frac{L}{2}) + M_A$$

$$M_A = wL^2/2$$

Entire structure:

$$\sum F_x = 0 = A_x + D_x - 1.5Lw$$

$$0 = A_x + 0.75Lw - 1.5Lw$$

$$A_x = 0.75Lw$$